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## MEMORANDUM

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<b>To:</b>	Gary Miller and Anne Foster U.S. Environmental Protection Agency	<b>Date:</b>	February 29, 2016
<b>From:</b>	John Laplante, P.E., Anchor QEA Wendell Mears, Anchor QEA David Keith, Anchor QEA	<b>Project:</b>	150557-01.01
<b>Cc:</b>	Phil Slowiak, IP David Moreira, MIMC		
<b>Re:</b>	Addendum 2, Operations, Monitoring, and Maintenance Plan San Jacinto River Waste Pits Time Critical Removal Action		

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The following provides Addendum 2 to the Operations, Monitoring, and Maintenance Plan (OMM Plan) for the armored cap installed as part of the Time Critical Removal Action (TCRA) at the San Jacinto River Waste Pits Superfund Site (Site). This Addendum 2 to the OMM Plan ("OMM Plan Addendum") is submitted on behalf of International Paper Company (IP) and McGinnes Industrial Maintenance Corporation (MIMC), referred to collectively herein as the "Respondents".

The purpose of the OMM Plan Addendum is to present a camera security system for the armored cap. This OMM Plan Addendum identifies the proposed locations and method of mounting of the cameras and procedures for conducting monitoring of the armored cap using the camera security system. It is contemplated that more detailed procedures for conducting monitoring using the camera security system and for maintaining and inspecting the system will be proposed following approval of this OMM Plan Addendum and installation of the camera security system.

The proposed security camera posts and anchor blocks will be positioned as shown in Figure 1. The locations for the posts and anchor blocks were selected to provide optimal views of the armored cap and surrounding areas without impairing inspection and maintenance events. The anchor blocks will be precast either at a location on the armored cap or at a location outside the Site and moved to the locations shown. The proposed cameras, power

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system, anchor blocks, and mast pipe for the camera security system are presented in Attachment 1. The structure is designed to withstand maximum wind speed of 110 mph in a three second interval. This design criteria is consistent with City of Houston building codes, which have been used as guidance since no other codes directly apply to this location. The drawings and sketches including in Attachment 1 illustrate the camera system, concrete anchor block, bolts and associated hardware, and mast pipe configuration for the proposed camera security system.

Each of the three proposed locations consists of two cameras powered by a battery and solar charging station. The paired cameras will provide 180 degree coverage, with the combination of the three locations intended to provide coverage of the capped area. The cameras will be operational 24 hours per day, 7 days per week.

The system utilizes video analytics to detect “motion” and is capable of remote monitoring and data retrieval. The cameras will be connected via a cellular network to a security monitoring firm. The monitoring firm will be alerted if motion analytics for any of the cameras are triggered. The monitoring firm will start notifying the Respondent’s using the emergency contact list within 30 minutes of the “alert” until they have made contact with a live person. Following review of the situation with the monitoring firm, the Project Coordinator or his alternate will notify the USEPA Remedial Project Manager and others, as needed, in accordance with the Site Emergency Response Plan (Anchor QEA 2016). The current emergency contact list is attached to this memorandum (Attachment 2).

## **Reference**

Anchor QEA, 2016. Emergency Response Plan, Prepared for U.S. Environmental Protection Agency, Region 6, on behalf of McGinnes Industrial Maintenance Corporation and International Paper Company. January 2016.

## **Figure**

Figure 1      Proposed Security Camera Anchor Block Locations

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## **Attachments**

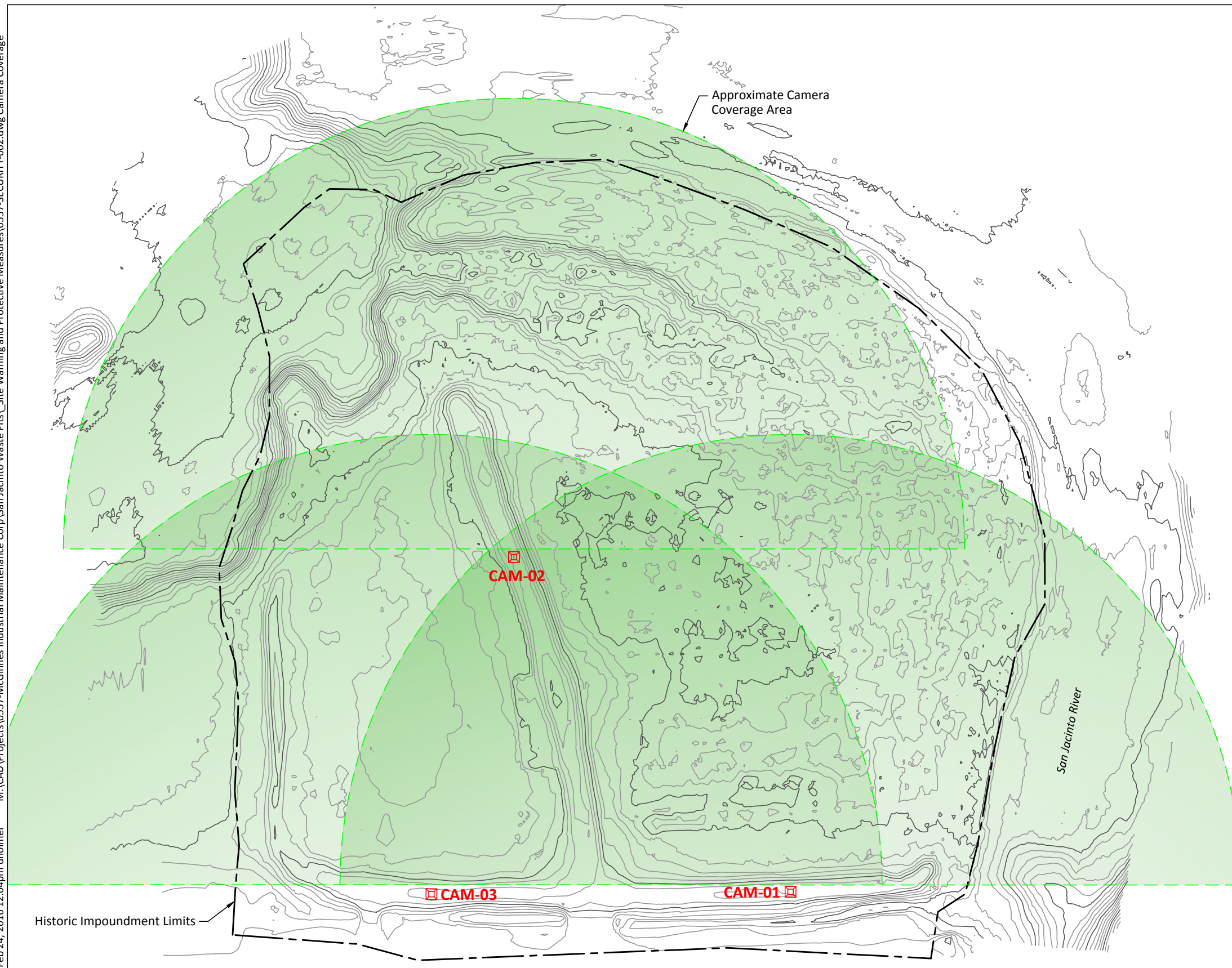
Attachment 1 - Specifications for Camera, Battery, Posts, and Anchor Blocks

Attachment 2 – San Jacinto Emergency Contact List

## FIGURE

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M:\CAD\Projects\0557-McGinnes Industrial Maintenance Corp\San Jacinto Waste Pits\_Site Warning and Protective Measures\0557-SECURITY-002.dwg Camera Coverage  
Feb 24, 2016 12:04pm dholmer



LEGEND:

- October 2015 Bathymetric and Topographic Contours (1 Foot Interval)
- Historic Impoundment Limits
- CAM-01 Proposed Security Camera Anchor Block Location
- Approximate Camera Coverage Area

**SOURCE:** Drawing prepared from survey provided by Hydrographic Consultants dated October 2015.  
**HORIZONTAL DATUM:** Texas State Plane South Central, NAD83, U.S. Feet.  
**VERTICAL DATUM:** NAVD 88.



0 120  
Scale in Feet

# ATTACHMENT 1 SPECIFICATIONS FOR CAMERA, BATTERY, POSTS, AND ANCHOR BLOCKS

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# ASHTON

## Engineering, Inc.

COASTAL & STRUCTURAL DESIGN

5867 Crystal Springs Ln., Bellingham, Washington 98226 Phone 360.354.8179

### CALCULATION SET

PROJECT NAME: AQEA San Jacinto Waste Pit Post & Anchors PROJECT: #21610.03  
PROJECT LOCATION: Houston, TX  
  
CLIENT: Anchor QEA  
Randy Mason, PE  
Huntington Beach, CA 92647  
657.227.7454

The design covered by these calculations posts supporting security cameras, solar panels, battery enclosures and concrete anchors. Only that part of the structure immediately affected by this work and shown or considered within this calculation set has been reviewed. Ashton Engineering is not responsible for any other part of the structure or any work being designed or performed by others which may impinge upon this work. Attachment of the above mention components to the posts is by others. The bolted connection at the post base is designed to fail at or above the design wind speed.

#### LOADS:

Wind: Per ASCE 7-10, V= 140 to 150 mph (3 sec) for Category I structures.  
Based on City of Houston design criteria use Basic Wind Speed of 110mph.  
Wind Exposure: D  
Seismic Design Category: A  
Soil Bearing Pressure: 1,500psf (Assumed clay, sandy clay, silty clay soil)

#### STRUCTURAL CALCULATION INDEX

<u>PAGE</u>	<u>REV.</u>	<u>ITEM</u>
1	-	Cover Sheet
2	-	City of Houston Criteria
3-4	-	Location and System data
5-12	-	Calculations
13-14	-	Drawings





## BUILDING CODE DESIGN CRITERIA

### CONSTRUCTION CODES

#### Commercial

2006 International Building Code  
2006 International Fire Code  
2006 Uniform Mechanical Code  
2006 Uniform Plumbing Code  
2014 National Electrical Code  
2009 International Energy Conservation Code, or  
ASHRAE 90.1-2007

#### Residential

2006 International Residential Code  
2009 International Energy Conservation Code

**Note:** All construction codes on this list have been amended by the City of Houston. To access the amendments, visit:  
<http://www.houstonpermittingcenter.org/code-enforcement/publications.html>

### STRUCTURAL REQUIREMENTS

Basic Wind Speed	110 mph (3-second gust)	Soil Class	Expansive*
Seismic Design Category	A	Wind Exposure Category	B*
Weathering Probability	Negligible (IRC)	Ground Snow Load	0
Roof Design Load **	IRC (20-lbs/sq. ft.)	Frost Line Depth	6 inches
Typical (Equation 16-27)	IBC ( $L_r = L_o R_1 R_2$ )	Maximum rainfall rate	8 inches/hour

\* These are general requirements for Houston and certain conditions may vary depending on the location.

\*\* Minimum roof loads shall be designed for the specific conditions in accordance with IBC Sections 1607.11.2.1 through 1607.11.2.4; and IRC Section R301.6, and Tables R301.6 or R301.2 (1) whichever is greater.

### ENERGY REQUIREMENTS

Heating Degree Days 1371 Climate Zone (Energy) 2-A

### PLUMBING REQUIREMENTS

Minimum Plumbing Fixtures See Table 2902.1 (of the Houston Amended Building Code)

### OTHER GENERAL REQUIREMENTS

Designs for Parking lots, Sidewalks and Driveways See Chapter 31 of the Building Code  
The Life Safety Ordinance for existing buildings Appendix L of the Building Code

### PLAN REVIEW REQUIREMENTS

- 2 Sets of plans
- Energy Code Form or Applicable Software Report (<http://www.houstonpermittingcenter.org/code-enforcement/publications.html>)
- TDLR numbers for accessibility (<http://www.license.state.tx.us/ab/ab.htm#techinfo>)
- Asbestos Survey for existing building(s) – (TDH Toxic Substances Control Division • (800) 572-5548)

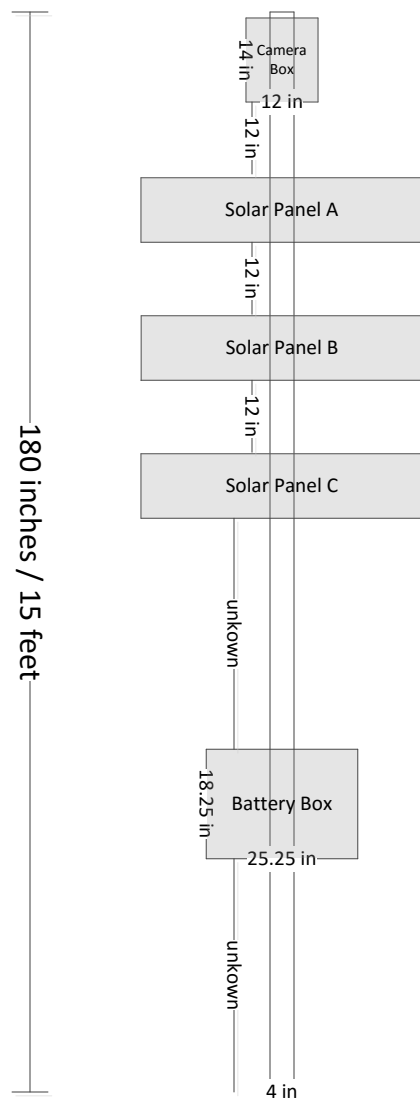
**Note:** This information is general in nature; for minimum submittal requirements, you may obtain a copy of the prerequisite checklist online or at the CACD office. For specific questions, consult with a Senior Plan Examiner (832) 394-8810 or Senior Inspector (832) 394-8840.

### WEBSITE INFORMATION

Check plan review status and schedule inspections at - <http://www.houstonpermittingcenter.org/city-of-houston-permits/online-permits.html>  
Code Enforcement/Code Update website - [www.houstonpermittingcenter.org/code-enforcement.html](http://www.houstonpermittingcenter.org/code-enforcement.html)  
Public Works and Engineering website - [www.publicworks.houstontx.gov](http://www.publicworks.houstontx.gov)  
Planning and Development website - [www.houstonplanning.com](http://www.houstonplanning.com)  
Fire Department website - [www.houstonfire.com](http://www.houstonfire.com)  
City Website - [www.houstontx.gov](http://www.houstontx.gov)

For questions please contact Lisa F. Brown at 832-394-9039 or email at - [lisa.brown@houstontx.gov](mailto:lisa.brown@houstontx.gov)





Above drawing is to scale using Mechanical  
Engineering 1/32 : 1

Weatherproof NEMA 4X Rated Enclosure for PPT5k  
Camera Unit



"The aluminum battery enclosure provides superior corrosion resistance and heat dissipation. They are light weight combined with strength and durability make them ideal cabinets for applications throughout many industries. They perform well in harsh, high heat, high wear environments." datasheet



Battery Box  
18.25"H x 25.25"W x 18.25"D

PPT5k Mobile Camera Unit:  
14"H x 12"W x 7"D

Solar Panel  
1.38"D x 21.5"W x 47"H

Pole Size  
Height Unknown  
4in Diameter if possible

WM Security Systems, Inc.  
Customer: San Jacinto Waste Pits  
Designed By: Justin Bolin  
Date: 01/25/2015  
Revision: 1

## AREAS

CAMERA BOX:  $(12 \times 14) / 144 = 1.16 \text{ SF}$

SOLAR PANELS (3):  $(3)(21.5)(47") / 144 = 21.05 \text{ SF}$   
(ASSUMING L TO WIND)

BATTERY BOX:  $(18.25)(25.25") / 144 = 3.2 \text{ SF}$

4"  $\phi$  (O.D.) PIPE:  $(4") \times (15' - 14\frac{1}{2} - 3(20.0\frac{1}{2}) - 18.25\frac{1}{2})$   
 $= 2.74 \text{ SF}$   $2A = 27.25 \text{ SF}$

TRY  $3\frac{1}{2}"$  SCH 40 (4"  $\phi$  O.D.)  
WALL = 0.226"

$S = 2.394 \text{ in}^3$  35 ksi

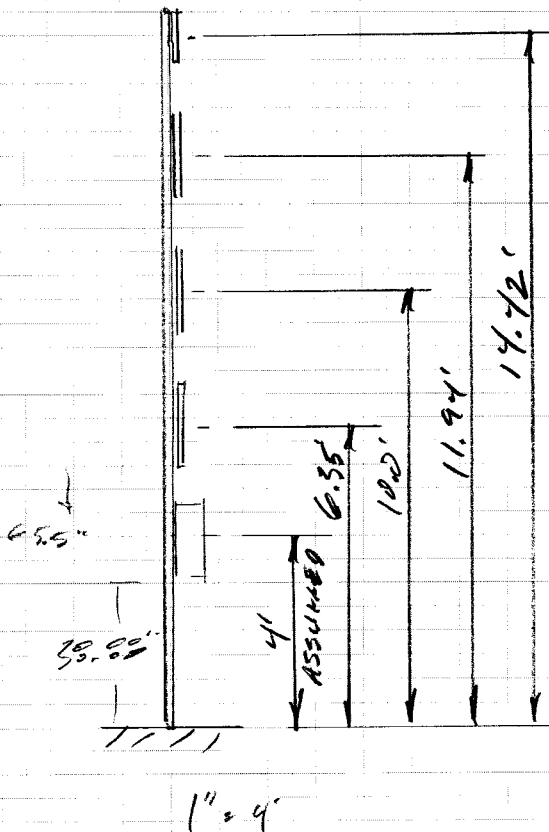
Moment = 4.61 k  $4/1.5 = 6.01 \text{ k}$

ROLLING CR: @ 20 PSI  
 $(20[1.10(14.42) + 21.05(10') + (2.74 \times 4)]) / 1000 = 4.8 \text{ k}$   
 $> 4.6$

4" SCH 40:  $4\frac{1}{2}"$  O.D.

MOM = 6.2 k

W/O DILATION  
INCREASE  
TYP.



TOP OF  
CONC BASE

## WIND LOADS PER ASCE 7-10

$V = 140$  TO  $150$  MPH (3 sec) CAT 1

HOWEVER, DESIGN FOR  $V = 110$  MPH 3 sec

WIND PRESSURE  $q_z = 0.00256 K_z K_{zt} K_d V^2$

$K_z = 1.03$  (EXP. 'D')

$K_{zt} = 1.0$  FLAT

$K_d = 0.85$

$q_z = 0.00256 (1.03) (1.0) (0.85) (110)^2 = 27.2 \text{ psf}$

ENR 29.5-1:  $F = q_z G C_f A_f$

$F = 27.2 (0.85) (1.65) (27.85 \text{ SF}) / \frac{1000}{1000} = \underline{1.06 \text{ k}}$

SECTION 20.9 } FIG. 29.5-1

TOTAL

RESULTANT HT OF AREA: (RECT. AREAS)

$\Sigma M = (14.42' \times 1.10 \text{ SF}) + (11.94' \times 21.05 \text{ SF}) + (4' \times 3.2 \text{ SF}) = 280.84 \text{ CF}$

$\bar{Y} = 280.84 / 25.41 = \underline{11.05 \text{ FT}}$

RESULTANT HT OF POST (4'4"):  $(10.6' \times 0.33 \times 3') + (5' \times 0.33 \times 0.67') + (1.62' \times 0.33 \times 3.24) = 13.33 \text{ CF}$

$\bar{Y}_{4'4"} = 13.33 / 2.25 \text{ SF} = \underline{5.95 \text{ FT}}$

## WIND LOADS, CONT'D

NOTE: WIND LOAD GOVERNS

$$\text{WIND}_{\perp} \text{ ON PANELS, ETC.} = 27.2(0.85)(1.05)(25.41 \text{ SF})$$

$$= 969.34 \text{ LBS}$$

$$M_{\text{BASE}} = (969.3 \text{ LBS})(11.05 \text{ FT}) = \underline{10.71'k}$$

$$\text{WIND}_{\perp} \text{ ON POST} = 27.2(0.85)(8.65)(2.28 \text{ SF})$$

$$= 34.26 \text{ LBS}$$

$$M_{\text{BASE}} = (34.3)(5.25) = \underline{0.20'k}$$

TOTAL MOMENT  
AT BASE = 10.91'k

$$V_{\text{TOT}} = \underline{1.0k}$$

$$S_{\text{REQD}} = 10.91'k(12)/0.66(35) = 5.64 \text{ in}^3$$

NOTE: 4" x 4" x 0.357" WALL (4 1/2" O.D.) S = 4.27 in<sup>3</sup>

ALL W/ 1.33 WIND OVERLOAD

$$= (4.27)(35)(0.66)(1.33)/12$$

$$= 10.93'k \checkmark$$

w/o OVERLOAD.

W/ A252 QMS,  $F_y = 45 \text{ ksi}$

HSS 4.5 x 0.337 S = 4.03

$$M_{\text{ALL}} = (45)(0.66)(4.03)/12 = 9.97'k \text{ w/o OVERLOAD, W/ 1.33 } M_A = 13.26'k$$

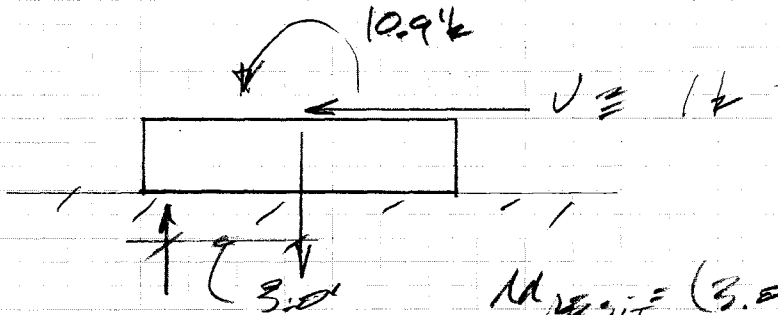
$$M_{\text{FIELD}} = 9.97/0.66 = 15.11'k$$

$$M_{\text{PLASTIC HINGE}} = (45)(5.50 \text{ in}^3)/12 = 20.62'k$$

USE HSS 4.5 x 0.337  
A252 QMS,  $F_y = 45 \text{ ksi}$

CONCRETE BASE

TRY  $4.5' \times 4.5' \times 18"$ ,  $WT = 150 pcf (61.38 CF) = 9,506 lbs$



$$M_{RESIST} = (3.0') (9.51k) = 28.5 k >> 10.9$$

TRY  $6' \phi \times 16"$ ,  $M_R = (2.75') (7.2k) = 19.8 k$

$$\frac{19.8k}{10.9} = 1.81 < 2$$

USE

TRY  $6' \phi \times 18"$ ,  $WT = 8.1k$

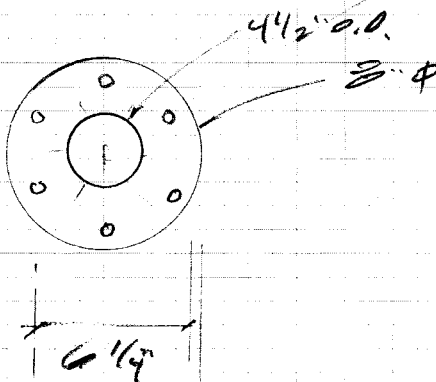
$$M_R = 22.28k \quad 2.07 \times M_R \quad \checkmark$$

$$BEARING AREA = (0.5') (6') = 3 SF$$

$$(3 SF) (1500) = 4500 lbs$$

$$(4.5k) (2.75') = 12.38k > 10.9k \quad \checkmark$$

BOLTED PLATE: 6 BOLTS @ 6" O.C.



$$\begin{aligned} V/\text{BOLT} &= \\ 1004 \text{ lbs} / 6 &= \\ &= 167.33 \text{ LB} \\ &(\text{LOW}) \end{aligned}$$

$$M = 10.9 \text{ k}$$

$$ARM = 6.25" = 0.521'$$

$$T/C = \frac{10.9 \text{ k}}{0.521'} / 2 \text{ BOLTS}$$

$$T = 10.46 \text{ k/BOLT}$$

$$V = 167.3 \text{ LBS}$$

TRY 7/16"  $\phi$  BOLTS

A307

$$\text{TENSILE AREA} = 0.1063 \text{ in}^2$$

$$\text{TENSILE STRENGTH} = (60 \text{ ksi}) (0.1063 \text{ in}^2) = 6.38 \text{ k} \quad \underline{9.4}$$

PLATE 3/4"

$$S = (8" \times 0.75)^2 / 6 = 0.75 \text{ in}^3 \rightarrow M_{ALL} = 1.48 \text{ k}$$

$$\text{IF WIDTH @ } 6", S = 0.543 \text{ in}^3 \rightarrow M_{ALL} = 1.11 \text{ k}$$

ASSUMES  
NO POST  
< 10.9 k  
w/o  
ALLSSTERS

$$T_{ALL} = (60) (0.1419) =$$

(ASSUME TWO  
BOLTS GO INTO TENSION)

(A307  
GR A)

$$\text{TRY } 1/2" \phi \quad T_{ALL} = 8.514 \text{ k} \quad \text{3k OUT LEVER ARM.}$$

$$ARM = 8" \rightarrow T/C = \frac{10.9 \text{ k}}{0.667'} / 2 \text{ BOLTS} = 8.17 \text{ k}$$

7/8"  
6/16"  
5/16"

$$7/16" \rightarrow 8.44 \text{ k}$$

$$7/12" \rightarrow 8.72 \text{ k} \leftarrow \text{USE } 7/12"$$

2.64 TO EDGE

$$T = 8.56 \text{ k}$$



(800) 547-6758

sales@portlandbolt.com

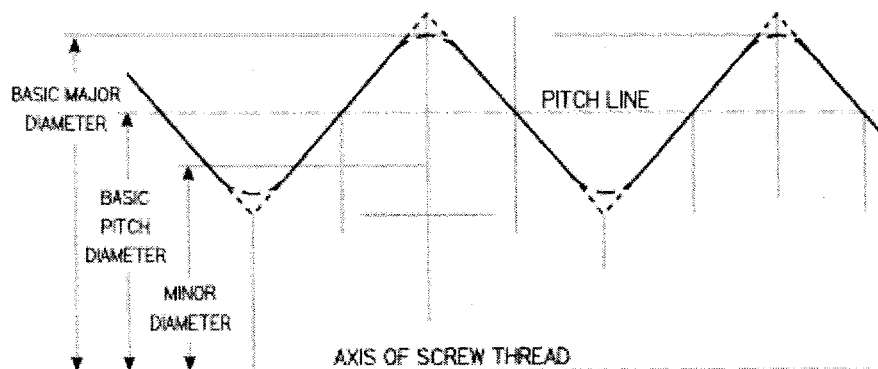
## Thread Pitch Chart

Thread series cover designations of diameter/pitch combinations that are measured by the number of threads per inch (TPI) applied to a single diameter.

**Coarse Thread Series (UNC/UNRC)** is the most common designation for general application bolts and nuts. Coarse thread is beneficial, because they are less likely to cross thread, more tolerant in adverse conditions and facilitate quick assembly.

**Fine Thread Series (UNF/UNRF)** is commonly used in precision applications. Because of the larger tensile stress areas, they have high tension strength. However, a longer engagement is required for fine thread applications than for coarse series threads to prevent stripping.

**8 - Thread Series (8UN)** is the specified thread forming method for several ASTM standards including A193 B7, A193 B8/B8M, and A320. This series is used for diameters one inch and above.



### Coarse Thread Series - UNC

### Fine Thread Series - UNF

### 8-Thread Series - 8UN

Nominal Size and Threads Per In.	Basic Pitch Dia. In.	Section at Minor Dia. Sq in.	Tensile Stress Area Sq in.	Nominal Size and Threads Per In.	Basic Pitch Dia. In.	Section at Minor Dia. Sq in.	Tensile Stress Area Sq in.	Nominal Size and Threads Per In.	Basic Pitch Dia. In.	Section at Minor Dia. Sq in.	Tensile Stress Area Sq in.
$\frac{3}{8}$ - 16	0.3344	0.0678	0.0775	$\frac{3}{8}$ - 24	0.3479	0.0809	0.0878	--	--	--	--
$\frac{7}{16}$ - 14	0.3911	0.0933	0.1063	$\frac{7}{16}$ - 20	0.4050	0.1090	0.1187	--	--	--	--
$\frac{1}{2}$ - 13	0.4500	0.1257	0.1419	$\frac{1}{2}$ - 20	0.4675	0.1486	0.1599	--	--	--	--
$\frac{9}{16}$ - 12	0.5084	0.162	0.182	$\frac{9}{16}$ - 18	0.5264	0.189	0.203	--	--	--	--
$\frac{5}{8}$ - 11	0.5660	0.202	0.226	$\frac{5}{8}$ - 18	0.5889	0.240	0.256	--	--	--	--
$\frac{3}{4}$ - 10	0.6850	0.302	0.334	$\frac{3}{4}$ - 16	0.7094	0.351	0.373	--	--	--	--

WELDS

$$0.93 \text{ k/in} / 1/16 \rightarrow \times 4 = 3.72 \text{ k/in}$$

$$CIR. = \pi d = 14.14" \quad \text{IF } 3/8", 5.55 \text{ k/in}$$

SAY  $2.5"$  OF WELD ( $3/8"$ ) =  $13.95 \text{ k}$  (USE 4455STS)  
IN TENSION

BASE PLATE, CONT'D

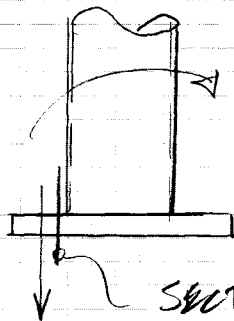
36 ksi:

CONSIDER AISC TABLE 15-12

$$a = 3/4", \quad b = 1.375" \quad \text{SAY } 1 1/2"$$

AVAILABLE TENSILE STRENGTH, k/in 8.03 k

$$\text{IF } b = 1 1/4", 9.7 \text{ k}$$



w/o ASSUMES

$$\text{SECTION } 7 1/2", \quad S = \frac{(7.5)(0.75)^2}{6} = 0.703 \text{ in}^3$$

$$M_{ALL} = \frac{(5.703)(56)(0.06)}{12} = 1.39 \text{ k} \text{ P.H.}$$

$$1" \text{ PLATE: } S = 1.25$$

$$M_{ALL} = 2.41 \text{ k}$$

$$\text{TRY GR 50 (A572)} \rightarrow M_A = 3.44 \text{ k}$$

USE 4455STS

USE GR 50 FOR BASE PLATE





tel 360.354.8179  
www.ashtonengr.com

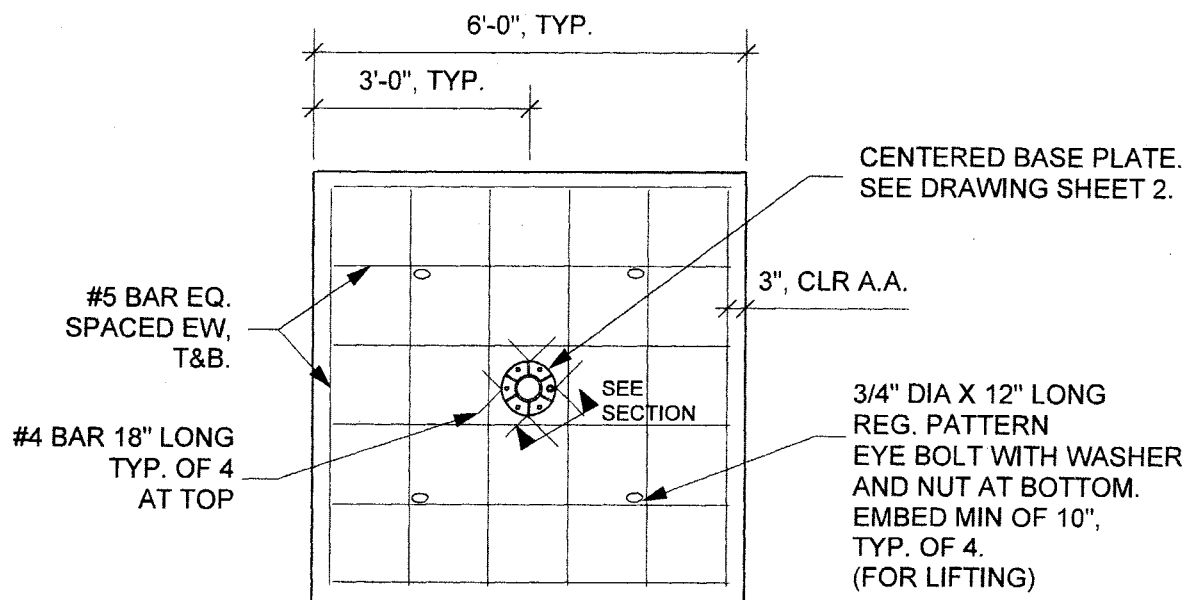
PROJECT AREA 105-  
PROJECT # 21610.03 PAGE 12 OF       
BY      DATE       
SUBJECT     

REL CHECK T/C @ 2 BOLTS

FROM ATTACHED DETAIL LEVEL  $A/H = 7\frac{5}{8}"$

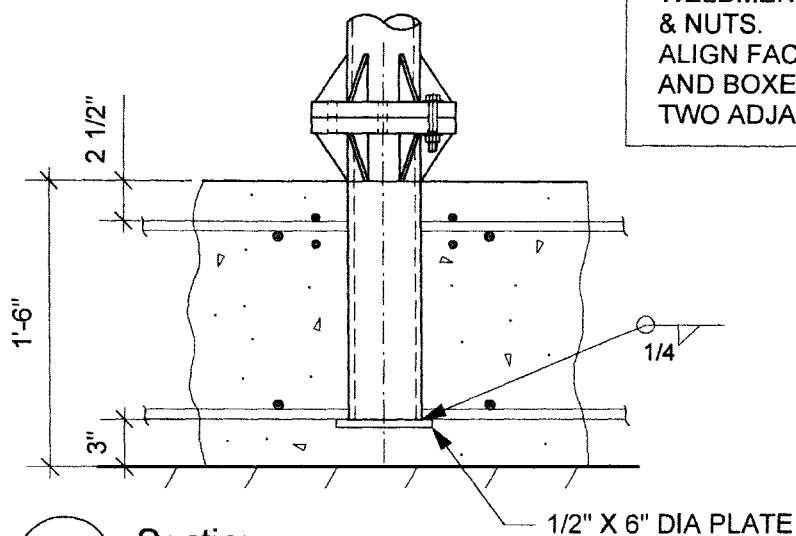
$$T/C = \frac{10.91k}{(7.625/2)} = 17.17k / 2 \text{ BOLTS} = 8.58k / \text{BOLT}$$
$$\approx T_u = 8.51k$$

✓ok  
(BOLTS FAIL)



**A** Plan - Concrete Base  
Scale: 3/8" = 1'-0"

**NOTE:**  
CONCRETE  $f_c$  = 3,500psi Min.  
60 ksi REBAR (BARE STEEL)  
HOT DIP GALVANIZE STEEL  
WELDMENTS, BOLTS, WASHERS  
& NUTS.  
ALIGN FACE OF SOLAR PANELS  
AND BOXES PARALLEL TO ANY  
TWO ADJACENT BOLTS.



**B** Section  
Scale: 1" = 1'-0"

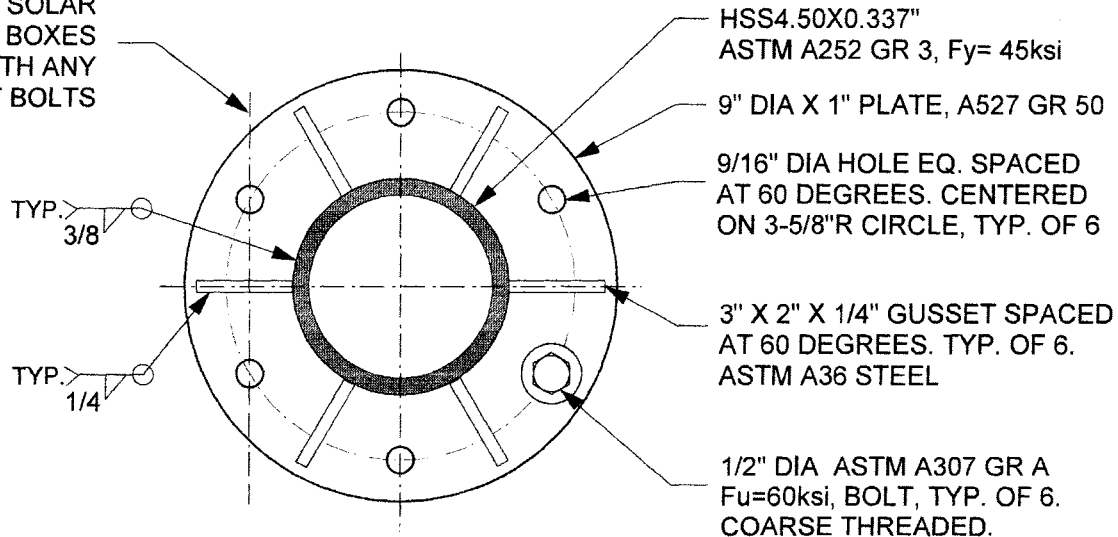


AQEA - San Jacinto Waste Pits  
Security Post and Anchor

JOB NO.: 21610.03  
SHEET 1 OF 2

9 FEB 2016

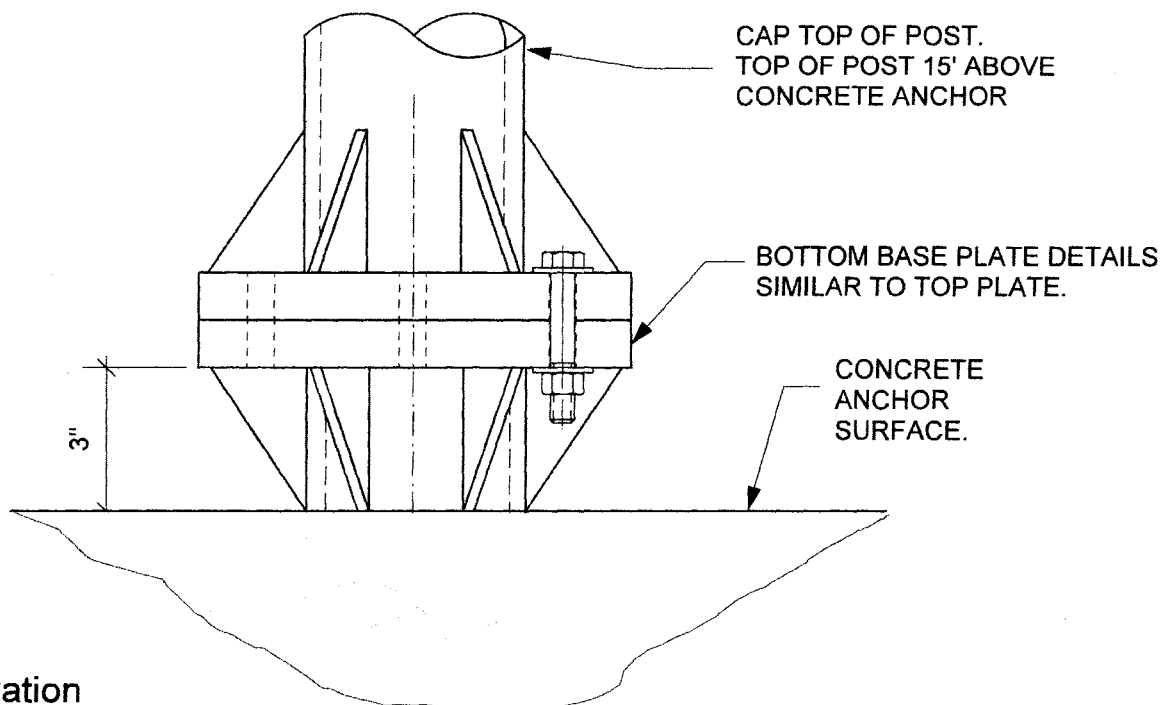
ALIGN FACE OF SOLAR  
PANELS AND BOXES  
PARALLEL WITH ANY  
ADJACENT BOLTS



**A**

Plan

Scale: 3" = 1'-0"



**B**

Elevation

Scale: 3" = 1'-0"



AQEA - San Jacinto Waste Pits  
Security Post and Anchor

JOB NO.: 21610.03

SHEET 2 OF 2

9 FEB 2016

## ATTACHMENT 2

# SAN JACINTO EMERGENCY CONTACT LIST

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## **SAN JACINTO EMERGENCY CONTACT LIST**

San Jacinto PRP Contractor: David Keith: 228.224.2983 (dkeith@anchorqea.com)  
Alternate: John Laplante: 206.795.2676 (jlaplante@anchorqea.com)  
Site Contact: Ed Fendley: 281.686.9199 (efendley@usaenviro.com)  
Alternate: Cesar Garcia: 832.641.0265 (cgarcia@unaenviro.com)

### **Scenario:**

#### **Significant Damage to Armored Cap**

- Emergency Response: \*911
  - EPA Emergency Response: 866.372.7745
  - Superfund Hotline: 800.533.3508
- Federal: National Response Center: 800.424.8801
- EPA Superfund Contact:
  - EPA RPM- Gary Miller: 214.542.9617 (miller.garyg@epa.gov)
  - R6 Remedial AR/TX: Carlos Sanchez: 817.395.3804 (sanchez.carlos@epa.gov)
  - R6 Remedial Manager: John Meyer: 214.665.6742 (meyer.john@epa.gov)
- State TCEQ Contact:
  - Monica Harris: 512.660.1120 (monica.harris@tceq.texas.gov)
  - Craig Watts: 512.731.2604 (craig.watts@tceq.texas.gov)
- Harris County Contact:
  - Bob Allen: 832.493.2232 (rpallen@hcphe.org)
- Port of Houston Contact:
  - Linda Henry: 832.287.4397 (lhenry@poha.com)